REVIEW

Emerging Challenges for Numerical Simulations of Quasi-Static Collision Experiments on Laser-Welded Thin-Walled Steel Structures

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Abstract

This paper re-evaluates recently published quasi-static tests on laser-welded thin-walled steel structures in order to discuss the fundamental challenges in collision simulations based on finite element analysis. Clamped square panels were considered, with spherical indenter positioned at the mid-span of the stiffeners and moved along this centerline in order to change the load-carrying mechanism of the panels. Furthermore, the use of panels with single-sided flat bar stiffening and web-core sandwich panels enabled the investigation of the effect of structural topology on structural behavior and strength. The changes in loading position and panel topology resulted in different loading, structural and material gradients. In web-core panels, these three gradients occur at the same locations making the panel global responses sensitive for statistical variations and the failure process time-dependent. In stiffened panel with reduced structural gradient, this sensitivity and time-dependency in failure process is not observed. These observations set challenges to numerical simulations due to spatial and temporal discretization as well as the observed microrotation, which is beyond the currently used assumptions of classical continuum mechanics. Therefore, finally, we discuss the potential of non-classical continuum mechanics as remedy to deal with these phenomena and provide a base for necessary development for future.

Keywords Collision and grounding · Experiments · Thin-walled structure · Finite element analysis · Welding

1 Introduction

Ship structural design for collision and grounding accidents has gained increasing interest during last decades. These accidents are related to highly non-linear structural behavior, which can be assessed experimentally, numerically, and in some cases analytically. Despite of several analysis methods,

Article Highlights

- Quasi-static collision tests on steel stiffened and sandwich panels are explained in detail for different failure modes;
- It is shown that simultaneous gradients on load, structural, and material gradients may cause rapid failure in panels;
- Microrotations are observed at the laser welds when exposed to pure shear or tension.
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there is still uncertainty on the true collision behavior as experiments in full-scale are extremely expensive. Thus, there is a need for reliable theoretical models, which predict the structural behavior correctly by including different mechanics for failure process and energy absorbing. In essence, the assessment consists of scenario definition (e.g., ship size, speeds, headings, structural, and loading conditions); assessment of internal (structural mechanics) and external mechanics (motions) during the collision event and assessing the consequences of the event (e.g., flooding, structural integrity, evacuation). In this framework, the current paper is limited to the internal mechanics and emerging challenges for numerical simulations. The main idea is to highlight the fact that although numerical tools are developing rapidly to assess the entire collision and/or grounding event, there remains several questions to be answered before we can use these tools with confidence.

Full-scale experiments with floating ships were first carried out by Vredeveldt and Wevers (1992) and Ohtsubo et al. (1994). The value of these experiments is the fact that they were carried out with realistic loading condition, i.e., hydrodynamic effects were combined with structural deformations



in order to have the correct energy balance of the collision event. Realistic experiments are possible in well-defined model-scale test, with the difference that only some of the effects can be included in the models due to similitude issues (e.g., Tabri et al. 2008; Calle et al. 2020). The difficulty of these kinds of experiments is the non-linearity of the phenomenon and large number of affecting parameters (e.g., contact location, material, geometry). Therefore, these tests include uncertainty on the details of the event as different effects evolve at different temporal and spatial scales. For instance, as shown for example by Ehlers et al. (2012), Werner et al. (2014, 2015), Wadley et al. (2013), and Jones (2013), the welds can fail in collision experiments, and this weld failure has significant effect on load-carrying capacity. However, often after the experiment the detail of failure process and time of weld failure remains uncertain. This failure information is however important when experiments are compared with simulations (for such comparisons see for example, Simonsen and Törnqvist 2004; Alsos and Amdahl 2009, Alsos et al. 2009; Ringsberg 2010; Hogström and Ringsberg, 2009; Woelke and Abboud 2012; Jones 2013; Woelke et al. 2013; Körgesaar et al. 2014, Kõrgesaar et al. 2017, Kõrgesaar et al. 2018a, b). Another important aspect of experimental work is the actual damage size (i.e., fracture-induced opening size) of the ship structure affecting the stability of the vessel and possible leakage of cargo (Ringsberg 2010; Schreuber et al. 2011; Hogström and Ringsberg 2012). Moreover, because of the stochastic nature of the waves post-accidental strength is of concern. It is well known that the ultimate strength of the ship is considerably reduced after the collision or grounding (e.g., Pedersen 2010; Guedes Soares et al. 2009; Sumi 2019). The reduction of the ultimate strength is naturally the function of the amount of plastic deformation and structural integrity after accidents. The weld failure, joining stiffener to the plating, causes so-called "shadow" effect whereby longitudinals are physically intact, but they do not contribute to longitudinal strength, particularly in the case of compressive loading.

Finite element (FE) simulations of ship collision and grounding are a standard tool for scientific and engineering research (Jones 2013). FE modeling can have different levels of sophistication. The rapid Equivalent Single Layer (ESL) models (e.g., Rabczuk et al. 2004; Romanoff and Varsta 2007) are based on the idea that the structural behavior is completely defined through a single geometrical plane in which the displacements and the stress resultants are evaluated. The challenge of these models is the integration of the constitutive law during non-linear collisions event than defines the relation between displacements and the stress resultants (see Figure 1A). The most commonly used models are based on the 3D geometry of the structure made from shell and weld elements (Ehlers et al. 2012; Werner et al. (2014, 2015)) and in some cases the computationally intensive but very accurate models made from solid elements on component

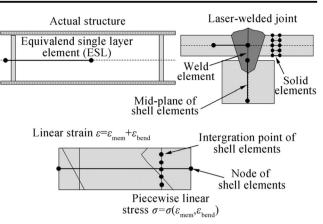


Figure 1 Structural idealizations commonly used in FE modeling from ESL shell towards full solid model and kinematics and stresses predicted by shell elements

level (see Figure 1A). For thin-walled structures, the shell elements are most common as they assume certain throughthickness behavior in terms of displacement and strains and use the non-linear constitutive expressions to compute the stresses and stress resultants (e.g., Woelke et al. 2013; Costas et al., 2019). These stress resultants (internal forces) balance with the external loading caused by structural deformations. Thereby, FE analyses allow flexibility, accuracy, and efficiency in terms of design as it can handle complex geometry, material behavior, and contact mechanics.

Due to the increase in computational capabilities, the FE simulations have been performed with ever-increasing mesh density. This allows modeling of finer details in the failure process than before. However, there is a size limit to which physical phenomena one should focus with a certain type of elements. For instance, the shell elements often follow the socalled first-order shear deformation or Kirchhoff plate theories, which assume linear longitudinal normal strains through the thickness of the element. This enables positioning nodes only in one plane of the element (see Figure 1B). This plane stress assumption means that the element cannot model through-thickness normal stress explicitly. In collision and grounding simulations, this through-thickness normal stress component is important from two perspectives (e.g., Costas et al., 2019). Firstly, during crushing and severe bending, the plate element exhibits high through-thickness stress at the contact side while at the tensile side this stress is close to zero due to lack of contact. Secondly, ductile fracture of steels is preceded by severe thinning or necking, implying significant development of through-thickness stress. In both situations, the stress triaxiality changes rapidly within the element. This triaxiality is in turn known to affect the ductile fracture significantly with the element size (see for example Wierzbicki et al. 2005; Barsoum and Faleskog 2007; Ehlers and Varsta 2009; Li and Wierzbicki 2010; Choung et al. 2012; Lou et al. 2012; Haltom et al. 2013; Körgesaar and Romanoff 2013; Dunand and Mohr 2014; Kõrgesaar and Romanoff 2014; Walters



2014: Östlund et al. 2015). Important second issue with the shell elements is also that they usually have several integration points through the thickness and the kinematics of the element is fixed. As a result, the strains in the element are forced to follow a linear through-thickness distribution, while the longitudinal stresses can be only piecewise based on the integration point values (analogy to laminates). This piecewise behavior is not realistic for homogenous materials as the stresses in reality are smooth over the thickness. The effect of this piecewise behavior becomes significant in bending dominated problems. Thus, to understand the strain and stress development in large structures, experiments are needed that characterize the failure process in detail so that the FE analysis gets information on the acceptable level of idealization on both in strains and stresses through the thickness of elements. Furthermore, the experiments should include both membrane and bending actions (e.g., Woelke et al. 2013; Costas et al., 2019). As stresses are impossible to measure in such situations, hybrid methods would be needed where experiments (determination of strains) are complemented with numerical simulations (computation of stresses based on measured strains). In last decade, the Digital Image Correlation has become an effective tool to characterize the experimental behavior of materials, components and structures undergoing large strains (e.g., Ehlers and Varsta 2009; Ehlers et al. 2012; Hogström et al. 2009; Hoogeland and Vredevelt 2017; Kõrgesaar et al. 2018a). This strain information is essential close to the point of fracture as it allows receiving more information about localization of damage and, for example, in situ measurements of the strain state when damage happens. This is important as the history of deformation plays a crucial role in determining the point of fracture (e.g., Uppaluri et al. 2011; Frodal et al. 2020). This is also important when there are gradients in material distribution due to welding or there are hard spots in the structure due to stiffeners, i.e., stiffness gradients locally in the structure. It has been shown by several experimental and numerical papers that optimization of geometry, materials, and manufacturing process (e.g., laserwelding) of the ship structures, can result in improved crashworthy structural designs (e.g., Naar et al. 2002; Fleck and Deshpande 2004; Rubino et al. 2006; Tillbrook et al. 2007; Ehlers et al. 2012). However, the changes in materials and structures in addition to variation in loading and boundary conditions in the structure make the prediction of the nonlinear phenomena at collisions and grounding event still a challenge for FE analysis.

Motivated by above mentioned experimental and theoretical findings from open literature, this paper collects and reevaluates the experimental findings made by authors during systematic experimental campaigns (Romanoff et al. 2006; Jutila 2009; Kõrgesaar et al. 2016; Kõrgesaar et al. 2018a, b; Kõrgesaar et al. 2019) for laser-welded thin-walled structures. The findings are reflected to the FE analysis ship collisions

and groundings as performed today. The main contribution of the paper is to present the situations, where the currently used main assumptions and simplifications of the Finite Element Analyses are becoming questionable. Furthermore, the usage of increasing Finite Element mesh densities and the future directions of development needs are discussed. Due to complexity of the problem, the paper is limited to internal mechanics of ship collisions and groundings, quasi-static conditions and flat panel structures to highlight relevant issues better. Two types of panels are considered. The stiffened panel structure made from normal strength steel with flat bars used as stiffeners. This structure represents the basic structural unit used in ship and offshore structures. The panel contains material gradient due to welds and structural (stiffness) gradient due to unidirectional stiffeners. The web-core steel sandwich panel is selected as the second structure, representing a double bottom or side structure of a ship. This structure has an additional structural gradient due to the rotation stiffness of the webs. The scantlings of the web-core panel are selected in the way that the initial in-plane, membrane, stiffness at zero load level is equal with the stiffened panel. Due to the geometry, the bending stiffness will be much higher in web-core sandwich panel, but the membrane behavior is assumed to dominate in large deflections. Loading position is varied in order to have gradient in out-of-plane shear forces of the panels. Due to these effects, experiments were carried out at panels, base and weld materials, and observations were made based on visual and audio observations and sensor signals. Thus, the focus of these experiments is to identify roles of loading, structural and material gradient to the non-linear response of thin-walled laser-welded steel structures. For consistency of the communication, the word "gradient" is used here to describe the sudden changes in distributions of an internal load of the statically indeterminate plate structure, structural stiffness changes due to stiffening and material properties due to laser-welding.

2 Panel Experiments

2.1 General and Test Matrix

Stiffened and sandwich panels were selected to model the two typical structural elements seen in shipbuilding (Table 1). Stiffened panels represent the most typical structural unit in the ship structures. The web-core sandwich panel is similar to the double bottom and side structures used as barriers for collisions and grounding. It also presents a family of crashworthy sandwich designs as indicated by scientific literature (Fleck and Deshpande 2004; Rubino et al. 2006; Tillbrook et al. 2007; Ehlers et al. 2012). Both stiffened and sandwich panels are laser-welded, resulting in similar welds and due to this, they have similar material gradient. Moreover, both



Table 1 The scantlings of the stiffened and sandwich panels used in experiments

Panel	Face plate (mm)	Web plate (mm)	Load location
Sandwich 1–3	1.5	3 × 23	Center
Sandwich 4			Edge at web
Sandwich 5			Edge between webs
Stiffened 1-4	3	3×30	Center
Stiffened 5			Edge

panels have the loaded faceplate stiffened by the webs that provide vertical support to the loaded face. This forms the structural gradient, which is similar in terms of vertical support, but considerably different in terms of rotation support to the faceplate. In web-core panel, due to the closed structure, the webs introduce significantly higher rotation support to the panels. In both panels, the loading position is changed to have different load gradients in the format of out-of-plane shear force. This load gradient is due to the statically indeterminate plate structure where the load sharing is defined by the stiffness along and transverse to stiffener directions. The details of the experiments can be found from Kõrgesaar et al. (2016); Kõrgesaar et al. 2018a, b; Kõrgesaar et al. 2019).

2.2 Panel Experiments

Material of specimens is standard structural steel S235JR (SSAB) with nominal values for mechanical properties as defined in standard EN 10025-2 (uniaxial test): yield strength = 235 MPa (280 MPa and 295 MPa measured for lower and upper yield limit respectively), ultimate strength = 360... 510 MPa (370 MPa measured) and elongation after fracture of 26%. The panels are manufactured from plate thicknesses 1.5 mm and 3 mm by Koneteknologiakeskus in Turku, Finland. Geometry and theoretical dimensions of panels are shown in Figure 2. Sandwich panels consist of face plates connected to each other with stiffeners. Stiffener spacing is about 120 mm for both panels. Faceplates and stiffeners are welded together with laser stake welds.

The quasi-static indentation experiments were conducted at the Strength of Materials laboratory of Aalto University. Hydraulic force cylinder with capacity of 1 MN was mounted to the loading frame (see Figure 3). Indentation force was measured with 1 MN force transducer connected to bottom end of the force cylinder. Sphere indenter was mounted on force transducer. Indenter could rotate during indentation to protect force sensor from moderate bending moments.

A clamping and support design configuration allows easy assembly and removal of the specimens and is based on bolted connection (see Figures 2 and 3). A square panel is fully clamped between two support plates made from standard

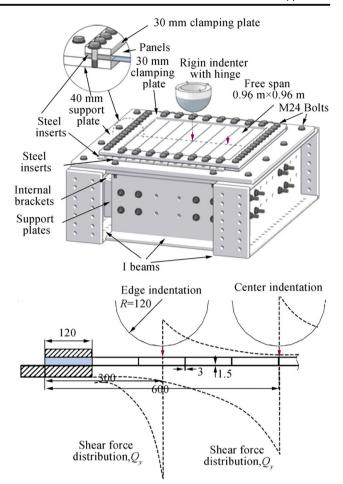


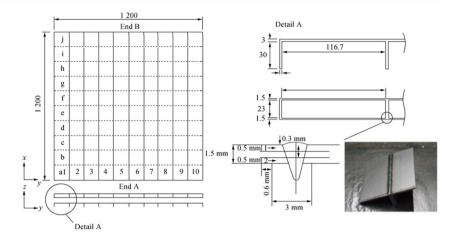
Figure 2 Test setup for panel experiments with the edge and center indentation resulting in different load gradients for out-of-plane shear force (modified from Kõrgesaar et al. 2016)

structural steel S355. The 40 mm thick back-support plate was bolted to an I-beam frame resting on a concrete support base. A 25 mm diameter, 62-hole pattern with internal threads in a back-support plate allowed edge clamping the test panels with 30 mm top clamping plate. Edge clamping was M24 bolts and tightening torque of 800 Nm resulting in a total clamping force of ~9 MN. Frictional clamping width in all edges was 120 mm resulting in a 960 × 960 mm unsupported panel spans. Steel inserts were used as supports inside the 120 mm clamping edges. Height and width of the inserts was designed to ensure tight fit into the enclosed space. Ibeam support frame was built from HE600B (DIN 1025/EN 10034) fastened together with grade 8.8 M24 steel bolts using four corner brackets. Additional support plates were used between brackets to adjust the beams into correct positions. Details of the stiffness of the support system and relative movement of the panels during the experiments can be found from Kõrgesaar et al. (2018b).

The load was introduced at the mid-span of the panel in the direction of stiffeners, i.e., at x = L/2 = 480 mm resulting in initially symmetric shear force, Q_x , distribution along the



Figure 3 Panel dimensions with coding of the unit cells (Kõrgesaar et al. 2016)



length of the panel. In this centerline, the transverse position was altered between y = 3/16 L, y = 7/16 L, and y = L/2 to allow initially gradients in the shear force distribution, Q_{ν} transverse to the stiffener direction. This shear force is known to cause secondary bending in the unit cells of the sandwich panels, and therefore to expose the laser-stake welds to significant bending that reduces the strength of the welds (see for example Romanoff et al. 2006, 2007; Frank et al. 2013). It should be noted, that as the panels fail during the experiment in a specific manner, the load-carrying mechanism of the shear force will change and the initial assumptions of the amounts carried in x- and y-directions become invalid. Due to this and the non-linear von Kármán strains, also the membrane and bending loading will be affected. In stiffened panel, the membrane load-carrying mechanism activates at very low load levels in comparison to sandwich panels where the bending stiffness is significantly higher and resulting in bending dominated load-carrying mechanism.

Indenter displacement was measured with HBM WA500 displacement transducer mounted between piston and cylinder and the vertical deflection of the loading frame was measured with 10 mm HBM WA10 displacement transducer. The amplifier is modular MGCplus by HBM with a CP 32B communications processor, 16 AP01 connection boards and 16 ML55B amplifier plug-in modules. Signal was transferred to PC via DT9834 digital-analog converter. Amplifier system settings were managed with MGCplus Assistant software. Force cylinder was displacement controlled and movement was defined with V-shaped amplitude profile, with maximum displacement being 300 mm. The indenter velocity was 10 mm/min. The indentation was stopped in all experiments after fracture of the specimen. The unloading process was manual and not according to the V-shape amplitude defined above.

2.3 Results and Experimental Observations

For the stiffened panels, both force-displacement and energydisplacement curves are basically overlapping with each other

when center load location is considered, while sandwich panels show larger scatter in the results (see Figure 4). The stiffened panel behavior is predictable until point of fracture where some differences are seen on residual strength and on the fracture pattern (see Figure 5). In all cases, the fracture initiates at the base material and proceeds after this within the base material or through the weld and the stiffeners, i.e., through gradients in structural stiffness and material properties. It is also interesting that the welds remain intact and position of loading does not cause surprises on the structural behavior. Thus, the load and material gradients do not affect the load-carrying mechanism of the panels to a significant extent. The sandwich panels exhibit contrasting response. The panels show very similar initial stiffness. Opposite to the global deformation seen in stiffened panels, sandwich panels deform across a few unit cells (stiffener spacings) in shear-dominated bending accompanied by acoustic emission of micro-cracking in very early stages of loading. This is a result of the secondary bending induced by the shear force, Q_{ν} , opposite to the web-plate direction, which is results in unit cell warping. The acoustic emissions are followed by very sudden drop (rapid) in load displacement curve with rapid and loud sound, which are a result of the lower faceplate fracturing on almost its entire length just below the indenter with fracture being in weld or HAZ. The phenomena are unexpected as the tests were carried out under room temperature and in quasistatic loading. On the other hand, at these panels the load, structural and material gradients meet at the same geometrical locations. As the top face is loaded by spherical indenter, the tensile and shear loading without the effects of contact are higher in the lower faceplate. It should be also noted that in this case the shear force, Q_{ν} , in the loaded web is very high because only few unit cells take part of the load-carrying mechanism. Thus, the stresses and strains next to the weld are much more complex and localized in sandwich panels than in stiffened panels (see Figure 6) (Kõrgesaar et al. 2019). As the loading is moved closer to the edge, the behavior in sandwich panels on initial stiffness is similar to that seen in center



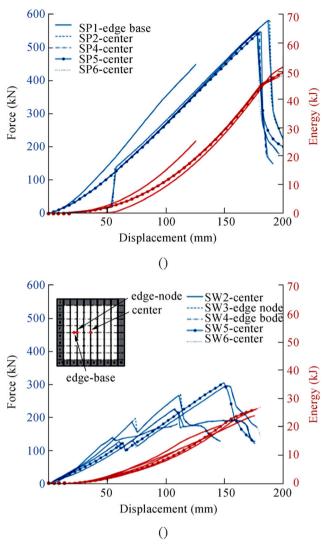
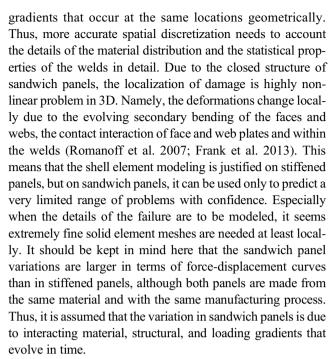


Figure 4 Stiffened and sandwich panel force-displacement and energy-displacement responses (modified from Kõrgesaar et al. 2016, Kõrgesaar et al. 2018a, b, Kõrgesaar et al. 2019)

load case, but the failure occurs earlier and in more dramatic manner, due to increased shear force, Q_y , gradient. This is seen as the decrease of the area below the force-displacement curve, which is an indicator of the energy absorbing capability of the structure. In a stiffened panel, such dramatic loss is not seen.

2.4 Consequences to the Global FE Modeling Uncertainties

The previous experimental observations mean that the discretization of the collision event in general requires very fine spatial and temporal discretization in sandwich panels, while in stiffened panels the discretization can be coarser. In addition, statistical information of weld properties in the case of sandwich panels must be included. The uncertainty of sandwich panels results from load, structural and material



In sandwich panels and in time scale, it is clear that some effects are extremely rapid and require high temporal resolution and incorporation of strain rate effects to the material modeling. The drops in force-displacement curves are dramatic, especially when the welds fail. As the failure progresses in a rapid manner and recreates a new load-carrying mechanism while progressing, the fine temporal scale is needed to capture these shifts. It should be kept in mind from experiments that the sounds of micro-cracking were followed by loud noise for final fracture, indicating that the temporal resolution should be in the order of speed of sound in steel.

3 Base Material Experiments

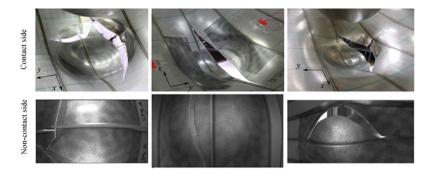
3.1 Test Setup

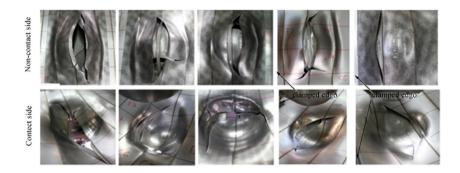
To gain more detailed insight to the base material behavior, a set of tensile tests were performed (see also for details Kõrgesaar et al. (2018a) and Figure 7). In these quasi-static experiments strain rates were not monitored.

A standard dog-bone (DB) specimen is common type to characterize standard material stress-strain behavior. The rest of the specimens were designed with use of 3D-FEA to obtain approximately constant stress states throughout the loading history until fracture initiation. The length direction of the specimens corresponds to the direction of stiffeners in panels the specimens were extracted from. A central hole (CH) specimen was used to characterize the material ductility under uniaxial tension (Roth and Mohr 2015). Stress triaxiality in the specimen remains nearly constant as confirmed with the preliminary FE simulations. Notched tensile (NT) specimen



Figure 5 Failure modes for stiffened and sandwich panels (modified from Kõrgesaar et al. 2016, Kõrgesaar et al. 2018a, b, Kõrgesaar et al. 2019)





was used to describe the state of plane strain. Preliminary FE simulations showed that notch geometry has significant effect on the stress triaxiality at the point of fracture initiation and iterations were needed to obtain the final shape in which the stress state remains constant. Shear (S) specimen design shown in Figure 7(d) was motivated by Roth and Mohr

(2015), and the shape of the specimen was a result of FE-based iterations as in the case of notched specimen. Quasistatic tensile tests were performed at room temperature with 3 mm thick specimens using a 100 kN MTS servo-hydraulic universal testing machine with an MTS Teststar Controller for displacement control. The crosshead velocity of the actuator

Figure 6 Stiffened and sandwich panel force-displacement and energy-displacement responses (reproduced from Kõrgesaar et al. 2018b)

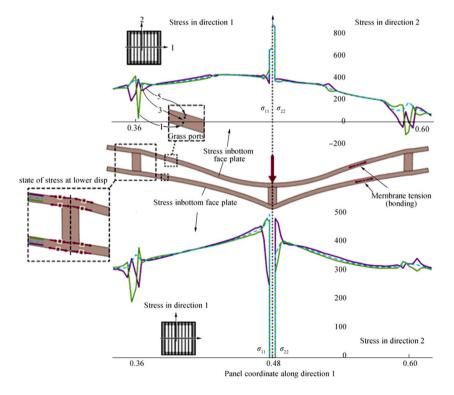
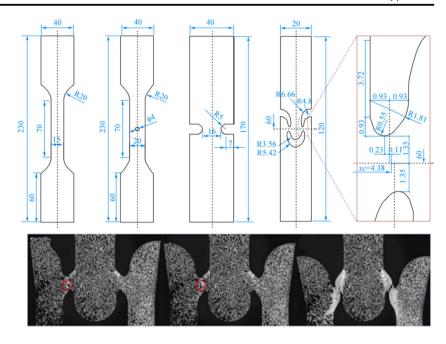




Figure 7 Tensile tests for base materials with different stress triaxialities (reproduced from Kõrgesaar et al. 2018a)



ranged from 0.2 mm/min to 2 mm/min, depending on the specimen. During testing, the force (using load cell) and displacement (MTS) were recorded along with the displacement fields on one side of the specimen surface were measured with a high-resolution (2 Mpxl) digital camera Lavision Imager Pro X 2M equipped with 105 mm Nikon lens at a frequency of 2 Hz. The images were post-processed using a LAVISION digital image correlation (DIC) software to acquire local displacement and strain data. It was observed that the speckle pattern produced by both printing and by paper separated from the specimens close to the point of fracture (see Figure 8). FE simulations are presented in Kõrgesaar et al. (2018a) and reviewed in Figure 8 to highlight the challenges in Finite Element simulations.

3.2 Results and Experimental Observations

The base material properties for different triaxialities are presented in Figure 8. The general observation is that the stressstrain curves per specimen type show good repeatability. All tensile experiments show localization of damage, meaning that we cannot neglect the necking phenomenon. Necking happens in the weakest section of the specimens, as it should. It is also clear from the experiments that stress triaxiality affects the entire engineering stress-strain curve, not just the softening part of the curve. The central hole and standard dog-bone specimen display diffuse type of necking with localization through the width of the specimen. The damage progressed gradually until the final fracture. Notched specimens display localized necking through-thickness. It is noteworthy that it is challenging to exactly define the location and time instance of the fracture onset because fracture initiates inside of the specimen where triaxiality is the highest and then grows towards the specimen surfaces. Moreover, in most of the specimens, the fracture propagation after onset shows rapid development except in the case of central hole specimen that exhibits significant softening. Thus, the sources of uncertainty in experiments are mainly due to variations in material properties, but also due to specimen shape that affects the failure strains.

3.3 Consequences to the Material Modeling Uncertainties in FE Simulations

These observations have a significant impact on the modeling of the stress-strain curves in the Finite Element simulations, as the models must incorporate changes in plastic, monotonically increasing part of the stress-strain curve as well as the softening part. In addition, as the experiments are based purely on membrane state instead of bending, the applicability of the material behavior in general becomes an open question in cases where the structural members of the panels are exposed to a significant amount of bending (see for example Woelke et al. 2018; Costas et al., 2019). This case rises in sandwich panel type of closed structures where the unit cells are exposed to warping (Romanoff and Varsta, 2007; Karttunen et al. 2019).

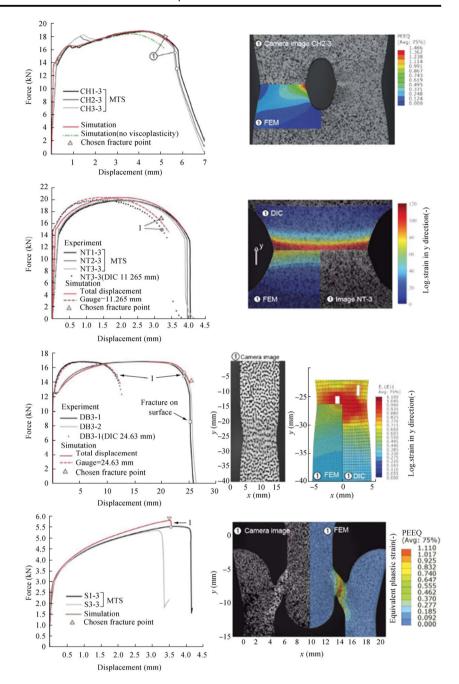
4 Weld Material Experiments

4.1 Test Set Up

Welds cause changes into material microstructure and can induce residual stresses to the specimens. These can have a significant effect on behavior of the structure in terms of



Figure 8 Tensile tests results and FE simulations for base materials with different stress triaxialities (reproduced from Kõrgesaar et al. 2018a)

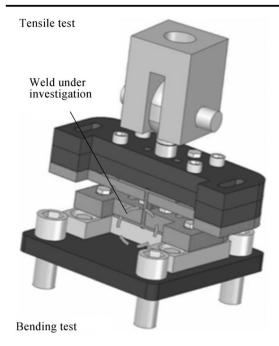


ductility. Therefore, standard Vickers hardness measurements were performed on stake weld cross-section to identify changes in material strength (Romanoff et al. 2006; Jutila 2009; Kõrgesaar et al. 2016) and the material gradient observed in the panels. Furthermore, the ultimate strength tests in tension (Jutila 2009) and bending (Romanoff et al. 2006) were performed in order identify the load-carrying mechanism and load-response behavior of the joints (see Figure 9). These two loading modes interact in the panels. Due to experimental limitations, the Digital Image correlation system was applied to the tensile ultimate strength experiment, while in the

bending experiment only the load and deformation were measured. In the hardness measurements, both 1.5 mm and 3 mm face plates and 3 mm and 4 mm web plates were considered. While the steel of the panel tests was S235JR, in ultimate strength tests of the welds it was S355J2G2 due to different manufacturer of the panels. This difference does not affect the qualitative experimental findings.

Tensile tests were done for welds cut from the sandwich panels and by strengthening the weld that is not under investigation with fillet welds (Jutila 2009). The supporting structure was based on 10 mm and 15 mm structural steel plates





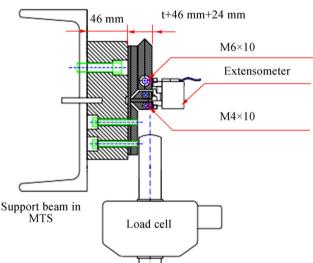
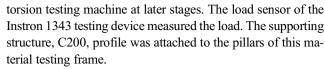


Figure 9 Tensile and bending experiments for laser-welds (modified from Romanoff et al. 2006; Jutila 2009)

that were supporting the faceplate so that it remains mostly undeformed and does not cause friction to the web of the loaded specimen. The force cylinder was Wärtsilä 45 860, the controlling displacement transducer HBM W50 and the force cylinder HMB 100kN U2B. In these tests, the Digital Image Correlation system of ARAMIS is used. The data acquisition system DAP2500a was by Microstar Laboratories.

In bending tests, the ultimate strength was measured by removing one of the faces of the sandwich panel and by restraining the bending deformations of both faces and webs by additional support plates attached to the specimen by M6x20 and M4x10 bolts. This allowed measurement of the displacement with extensometer INSTRON1343 at initial stages and by displacement sensor of the Instron 1343 axial-

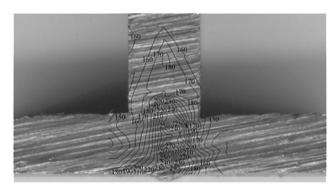


Both tensile and bending tests were performed by increasing the loading monotonically until the specimen broke or until the maximum measuring capacity of the system was reached.

4.2 Weld Geometrical and Material Properties

The hardness profiles of the specimens are shown in Figure 10 for the specimens from Jutila (2009) and Kõrgesaar et al. (2018a).

The profiles measured for different sets of specimens are very similar in general and show that the stake weld penetrates vertically to the web plate by more than the plate thickness is. However, the penetration is not for the full width of the web, meaning that there exist two crack-like defects at both sides of the weld. As can be seen from Figure 10 the hardness is within



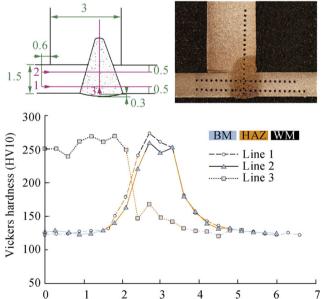


Figure 10 Hardness profile measured of laser-welds (reproduced from Jutila 2009; Kõrgesaar et al. 2018a)

Measurement location (mm)



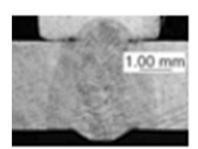
the limits of the classification societies and it have very rapid change between base and weld material since the heat-affected zone, HAZ, is less than 1 mm in width. This is an indication of a significant material gradient due to the welding process. The different type of weld positioning of different panels is shown in Figure 11. It is clear from this figure that the crack-like defects are not always symmetric with respect to the weld mid-line, indicating that the moment-curvature relation of the T-joint can be asymmetric due to a contact (e.g., Boronski and Szala 2006; Romanoff et al. 2007).

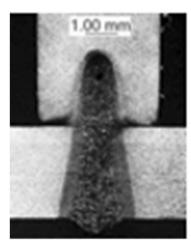
4.3 Results and Experimental Observations

The strength of welds in tension is highly localized in the heataffected zone (see Figure 12). It should also be noted that the notches at both sides of the welds do not act as the initial cracks leading to final failure, but rather as crack initiation points. This is due to fact that the differences in material properties over the weld interact with this geometrical feature. Thus, these notches act as damage localization sites, but not as indicators for direction of the crack path. As can be seen from the specimen the damage localizes there and microrotation, associated with shear, is seen to occur at the point of failure. This is an indication of non-locality in terms of classical and non-classical continuum mechanics (e.g., de Borst 1990; Bazant and Jirasek 2002, Srinivasa and Reddy 2017). The DIC pattern used is removed due to this just before the final fracture. It should also be noted that the response of the specimen is monotonically increasing until the fracture and the plastic region of the specimen is very small in comparison to the base materials. The bending tests show a different behavior (see Figure 13). There the force-displacement or moment-slope behavior is non-linear and significant plastic region is observed. Moreover, due to the different position

Figure 11 Different types of stake welds observed in panels (reproduced from Romanoff et al. 2006)

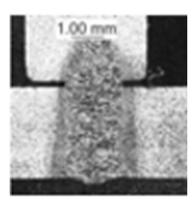
Center weld. incomplete web penetration

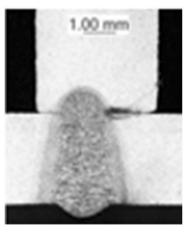




Center weld, very deep web penetration

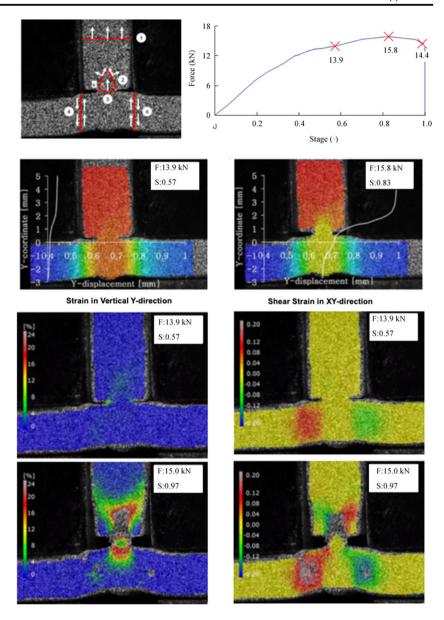
Center weld, significant root-gap





Off-center weld, insignificant root-gap

Figure 12 Tensile tests for weld materials (modified from Jutila 2009)



of the welds, the repeatability of the experiments is poor. This uncertainty is caused by contact which varies considerably between specimens.

It is clear that the differences are due to the possible contact at the neighborhood of the weld but also due to the different widths of the welds. Furthermore, in these experiments welds do not fail due to fracture, but form a plastic hinge. These observations indicate that although the unit cells warp in sandwich panel experiments and expose the welds to bending, the rapid micro-cracking at low load levels is not only the result of the bending but also the shear type of stress state that is more critical in sandwich panels than in stiffened plates. The reason can be that only a few unit cells deform in the sandwich panels while the stiffened panels deformed more globally. Due to this, the shear force, Q_x , along the stiffener is higher in sandwich panels, which can result in the rapid, almost catastrophic,

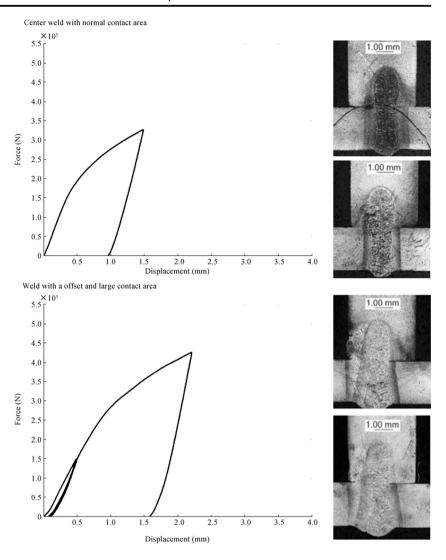
failure of welds. This in combination with high shear force, Q_y , makes the welds highly loaded. This rationalization of sandwich panel behavior is a clear indication of strong non-linear interaction between material, structural and loading gradients.

4.4 Aspects on Finite Element Modeling

The above observations suggest that welds and possible contacts should be modeled in great detail in FEA that poses a significant challenge (see Figure 13). This requires solid element mesh, which increases mesh size significantly, because we have thin-walled structure where some of the phenomena are truly 3D close to the welds. Another aspect is the microrotation at the welds. Current FE-codes based on classical continuum mechanics cannot capture these microrotations,



Figure 13 Bending tests for weld materials (modified from Romanoff et al. 2006)



which leads to mesh size dependency and "size effect" on the damage. This fundamental issue could be overcome by elements size-dependent weld strength criteria. However, as the base material experiments and simulations show, this is a challenge already for the homogenous base materials undergoing both membrane and bending actions. In welds, we need to consider in addition the heterogeneous material distribution, uncertainty in micro geometry, micro rotations and contacts making development of such criterion very challenging. Thus, there is need for properly working finite element based on non-classical continuum mechanics that allow material rotation and modeling of these effects.

5 Discussion of the Challenges Observed

The paper investigated experimental observations regarding the failure of quasi-statically loaded laser-welded stiffened and sandwich panels. We aimed to identify the structural and material failure modes both qualitatively and quantitatively. The paper aimed to highlight the challenges we have in finite element simulations and how to tackle those to increase the fidelity of simulations. The discussion was based on the experimental observations with panels investigated at the length scale of real structures, base materials, and welded joints. The focus was on the experimental classification of the phenomena based on loading, structural, and material gradients. We discuss each of these aspects next while reflecting on potential challenges related to numerical simulations; the summary is visualized in Figure 14.

The loading gradient was investigated by positioning the spherical indenter to different positions in a square panel. Theoretically, at the initial, zero load level, the panel response is defined by bending stiffness that includes both stiffness against bending moments and out-of-plane shear forces. As the panel is square, the geometry does not affect the loading gradient, but the orthotropy, position of the loading, and the boundary conditions will affect to a large degree. The role of the orthotropy is to define the share of loading carried out in principal, stiffener direction and transverse to it. In the panels



300 200 100

Experimental observation Numerical challenge Possible solutions Rapid panel failure and load-drops due to FE-discretization becomes function of Mesh refinements needed at the location of simultaneous load, stuctural and material localization of damage (space and time), simultaneous load, structural and material gradients (e.g. welds) which make the assessment of new This results in early shift of load-carrying structural designs a challenge in terms of Fine spatial and temporal discretization in mechanism from bending to membrane actions uncertainty FE-mesh Unified material model that accounts for At base material tests necking is almost Multiscale models for material and mesh size, stress triaxiality, hardening and impossibe to omit (violated plane stress strucutal strength modeling softening under combined membrane stretch assumption) and failure initiates inside the and bending in needed specimen (DIC provides only surface strains) Micro-rotations observed at the welds when Non-local solid elements needed at the Need to model the welds with contact and exposed to pure tension/shear and contates welds microrotation observed when in bending 600 A Force (kN)

10

200

Figure 14 Summary of the experimental observations, emerging challenges and possible solutions for numerical simulations of quasi-static collision experiments on laser-welded steel structures

selected, the orthotropy in shear stiffness is high, especially in the web-core sandwich panels. Due to this, significant part of the shear loads is carried in stiffened direction and significantly less in the opposite direction. As a result, the sandwich panels deform initially locally, activating only a few unit cells in the panel. This is highlighted by the fact that the sandwich panels have very high stiffness against bending and twisting moments, the panels deform mostly in shear in the weaker direction, opposite to stiffeners (e.g., Romanoff and Varsta 2007; Karttunen et al. 2019). This results in structural gradient due to stiffener rotation restraint. Due to structural gradient, the shear force, Q_{ν} , causes high secondary bending to the faces, webs and welds, and as a result, the sandwich panels start to damage at very small load levels locally. This damage is affected by the material gradient observed at the region where base material properties change to weld properties through heat-affected zone. In stiffened panels, due to smaller bending capacity, the panels experience earlier membrane action and therefore deform more globally. When loading is moved closer to the edge of the plate or away from the stiffeners, the shear force gradient increases considerably, and the failure starts earlier due to increased secondary bending. These phenomena are related to low load levels. As the panels deform at higher load levels, both of them deform under membrane actions. However, at this stage the local damage of sandwich panels has already spread and changed the loadcarrying mechanism considerably. From residual strength viewpoint, the mechanism is uncertain as the stiffeners have separated from the faceplates to some degree. This way they

100

Displacement (mm)

150

do not stiffen the plates anymore. Thus, if the collision or grounding damage is followed by in-plane compressive load induced by hull-girder bending, the stiffeners would not provide any more vertical or rotational support. In sandwich panels, the failure mode is a very rapid sequence of events, meaning that the temporal resolution of experiment and simulations should be much higher than in stiffened panels.

The orthotropy at the panel level results in complex damage accumulation at the level of base materials and welds. The stress-strain curves obtained from membrane loading state with different triaxialities show different plastic parts of the stress-strain curves, which are even more different during the softening stage beyond necking. The benefit of softening in FEA context is that the element removal is not sudden, and thus there is a possibility to redistribute the stresses when damage progresses (e.g., Abubakar and Dow 2013; Körgesaar and Romanoff 2013. These localized phenomena can be incorporated into effective stress-strain curves, but these should be modified by the element size, strain state and rate and stress triaxiality to reach good agreement between large-scale simulations and the damage. In addition, the models should be extended to stress states including compression and to account the effects of bending and its influence on the through-thickness strain and stress distributions on shell and solid elements. These issues could be dealt with multiscale modeling techniques of materials science, which allow direct simulation of material strength under complex loading scenarios (e.g., Geers et al. 2019; Matous et al. 2017). However, these techniques increase the computational tasks



significantly. The issue could be handled by parallel computing or by developing new models for material failure based on typical observed load paths (e.g., strain paths) from thinwalled structures. Additional complex damage component arose due to micro rotations associated with shear in the welds and heat-affected zone (HAZ). Current FE-codes based on classical continuum mechanics cannot models such effects and need to be extended to handle non-classical continuum mechanics formulations, which account for these higher-order deformations. The main benefit of these formulations is that the size dependency of the FE-solution is handled with much better convergence rates, giving a chance for engineers to model the complex phenomena with better confidence (e.g., de Borst 1990; Srinivasa and Reddy 2017).

The paper was limited by sample size and the fact that experiments were quasi-static. Despite the quasi-static loading, some observed phenomena were rapid and thus, the strain rates should be measured, especially during fracture onset directly from the surface of the structure and possibly also from inside. DIC was not successfully used in panel level, but worked out in material scale experiments, in 2D, as there are no curvature end deformations are uniform stretches. In cases of micro rotation or significant distortion, it is clear that better DIC patterns are needed, which stick on the specimen until fracture occurs and beyond. The experiments reveal that there is non-locality both in panels and in welds. ESL (e.g., Reinaldo Goncalves et al., 2019; Namaplly et al. 2019) and solid Finite elements with this effect are emerging and clearly would improve the predictions significantly. These aspects are left for future work, but it is clear that there is a need for these developments.

Ultimate residual strength and opening size are critical aspects in marine structures after collision or grounding has occurred. Present paper shows that the damage shapes are different in stiffened panels, although the force-displacement curves are practically the same to the point of fracture. In the case of sandwich structures, the differences are seen already at the forcedisplacement curves. This certainly affects the opening size and the flooding following the damage. The flow rates through damages in stiffened single skin panels and the double skin type sandwich panels are clearly a difficult flow problem that needs research for different opening shapes and sizes resulting from experiments. Another aspect is the residual strength after the damage. Carrying the compressive loads might be problematic for sandwich panels that have lost their structural integrity, in contrast to stiffened panels that are still intact in terms of faceplate to stiffener connection. Thus, the stiffeners are still providing strength against plate buckling, while in sandwich panels this assumption does not hold. This would affect the load-carrying capacity of the structure significantly. In tension, the complex, localized fracture patterns would lead to different global fracture processes of the structures. Both compression and tension would need further research on prevailing failure modes under these basic load conditions.

A recent study by Berntson et al. (2019) on sandwich panels shows that when welds are explicitly included in FEA based optimization with evolutionary algorithms, the optimized structure becomes insensitive to the material and structural gradients. Thus, a robust design is obtained that is less sensitive to these loading, material and structural gradients. If the optimization is done without the weld, the result is very sensitive to these gradients. This is a sign that modern computational optimization techniques are able to find wellperforming structures if the modeling is carried out in a right manner. This is, at the same time, a threat and an opportunity. A threat, as the designers may become over-confident with their designs and not checking the structures carefully for complex interactions between the structure, loading, manufacturing processes, scenario definition, and materials selection. It is also an opportunity as if the modeling is done correctly, and we understand the failure processes in-depth, we can leave the optimization for computers with the aim to find a robust, well-performing structure.

6 Conclusions

The aim of this paper was to review the lessons learned from quasi-static collision experiments made by authors on laserwelded steel thin-walled structures, with aim to highlight the challenges for numerical simulations based on Finite Element Method. The panels tested had to various degrees, differences in gradients of material distribution, structural continuity and loads or boundary conditions. These gradients were used to describe the changes in load-carrying mechanism of the panels and were shown to have a significant impact on the way the load is carried. The experiments show that the stiffened panels perform predictably and the force-displacement curve is monotonically increasing, while in sandwich panels, sudden drops are observed, and the behavior is less predictable. These drops are due to complex, interacting load-carrying mechanisms that consist of load, material, and structural gradients. It is also seen that the failure mode in welds localizes through material rotation, which is impossible to model with classical continuum mechanics in detail and instead non-classical continuum mechanics are needed. These observations indicate that there is room for better computational techniques that can handle both spatial and temporal scales more accurately and effectively than today.

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